# Research Report KTC-96-23

# Rubberized Asphalt Membrane

by

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Kentucky Transportation Center College of Engineering University of Kentucky

in cooperation with Kentucky Transportation Cabinet

and

Federal Highway Administration U.S. Department of Transportation

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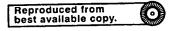
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# RUBBERIZED ASPHALT MEMBRANE

# **EXECUTIVE SUMMARY**

This document reports findings of a study involving the use of scrap tire chips in an asphaltic membrane. The research project was designed with two objectives in mind. First, to investigate the effectiveness of an asphaltic membrane on top of a subgrade for maintaining moisture equilibrium in subgrade, and second, to study the potential use of scrap tire chips in asphaltic membranes. The effectiveness of the membrane as a moisture barrier needs to be evaluated over a long period of time (i.e. several years). However, the method proved to be a feasible alternative for recycling waste tires in pavements. It is hoped that this study will contribute to various efforts in the area of cost effective and sound utilization of waste materials in construction.

#### INTRODUCTION

The use of an asphalt membrane interlayer in pavements is not a new concept. Researchers and practitioners have had an ample amount of experience with pavement membranes for stress relieving purposes (Kidd 1990, Moody 1994). Such a membrane is referred to as stress absorbing membrane interlayer (SAMI). The SAMI is often sandwiched between an old pavement surface and a new overlay (NCHRP 1989, Lorenz 1987).

The membrane interlayer in this project was designed to serve two diverse purposes. First, to investigate the effectiveness of an asphaltic membrane on top of a subgrade for maintaining moisture equilibrium in the subgrade, and second, to study the potential use of scrap tire chips in asphaltic membranes. Various uses of scrap tires in hot mix asphalt (HMA) have been explored in the past several decades, as summarized by FHWA (Heitzman 1992). In this study, it is hypothesized that the membrane interlayer applications with tire chips in pavements may not have as many potential environmental side effects as there are associated with the HMA-crumb rubber applications. Such problems may include: potential emissions problems during production of HMA, and issues related to re-recycling.

The initial concept of a membrane on top of the subgrade was presented in Kentucky by Mr. Ellis Williams (Williams 1989) as a means for maintaining moisture equilibrium in the subgrade. This theory continues as far as stating that marginal subgrades with potential for shrinkage and swelling due to moisture fluctuation may be "stabilized" through a moisture stabilization process.

Plastic clays are susceptible to expansion and shrinkage due to changes in their moisture content. Chemical modification (stabilization) of subgrade soils in highway construction is often a standard procedure for dealing with expansive clays. On the other hand, the membrane concept which has been put forward involves the use of a membrane to partially isolate the subgrade. This partial isolation of subgrade is hoped to provide moisture equilibrium, a condition which would theoretically render chemical stabilization unnecessary. It must be mentioned that chemical stabilization has the unique long-term advantage of pozzolonic cementing and subgrade strengthening properties, which are highly desirable. However, the advantage of applying membrane is in its expediency and ease of application.

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#### **EXPERIMENT DESIGN**

An experiment was designed to evaluate the construction and performance of the membrane in this study. The following presents a summary of parameters that have been included in the experiment.

Table 1. Summary of Experiment Design Parameters.

	Membrane	Non-Membrane
Base Layer Gage	Locations: 1,2&3	Locations: 1,2&3
Subgrade Gage	Locations: 1,2&3	Locations: 1,2&3

#### **Control Section**

In order to have a statistically valid comparison, a control section was included in the project (i.e. a section of the project was constructed without a membrane).

## Replication

Moisture sensors were installed at three locations along the length of the project. This level of replication was maintained for both the control and the membrane treated sections.

#### Moisture Sensor Placement

The effectiveness of the asphalt membrane as a moisture barrier must be evaluated on a long-term basis with moisture sensors. These sensors were placed under and over the membrane within the pavement structure at three replicate locations (that is, a total of six sensors). In order to allow for valid statistical comparisons, six sensors were installed in the control section as well. These were in an approximate symmetry to the section with the membrane along the opposing traffic lanes.

#### Minimizing Bias

Care was taken to select a relatively flat terrain section of the roadway for conducting comparisons. Also, visual inspections were made to ensure uniformity of geological features as well as absence of any major surface and/or underground water sources that may introduce bias into the pavement moisture profile.

#### Randomization

The selection of roadway approach for application of the membrane, as well as the selection of moisture sensors and their installation was conducted in accordance to a random process. This was done in order to ensure randomness of errors and minimization of bias.

# MEMBRANE CONSTRUCTION

# Project Location and Layout

The membrane project that is being discussed in this paper took place on State Route KY-9, approximately 3.2 kilometers (2 miles) west of Maysville, Kentucky -- or approximately 43 kilometers (55 miles) southeast of Cincinnati. Construction of the new pavement on this project was part of an expansion from two lanes to four lanes; each traffic approach was approximately 1.8 kilometers (1.1 miles) in length. The KY-9 state route is a major rural arterial along the Ohio River that connects the following cities: Cincinnati, Covington, Alexandria, and Ashland; this road is also known as the "AA-Highway" (connecting Ashland to Alexandria). A schematic layout of the project is given in Figure 1. Construction activities on this project were documented on photographs and videotapes, which will be maintained on file at the Kentucky Transportation Center.

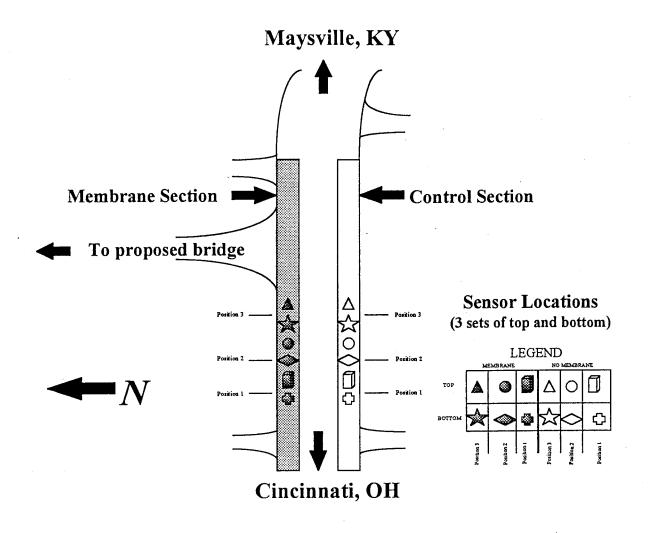


Figure 1. Project Layout.

# Construction Planning

The membrane design called for a rich spray application of asphalt cement directly on top of the subgrade. The design also included a stone coverage as well as scrap tire chips. At the first glance, the basic construction issues surrounding this project appeared to be similar to seal coat construction. These fundamental construction issues were outlined by several researchers (Benson and Gallaway 1953), and (Kearby 1953). However, this project posed unique issues that are presented in the following paragraphs.

Conceptually, the design of the membrane in this project intended for the mixture of tire chips and the asphalt to form a good contact and form a matrix. The entire membrane was to be covered with a conventional flexible pavement, details of which are presented in the section dealing with the pavement structure.

The following sections describe the process by which the constructibility issues were examined.

# Constructibility Issues

Initially, it was envisioned that the tire chips may be mixed with the cover stone to provide an aggregate blend (rocks and tire chips) for covering the freshly sprayed hot asphalt. The question was raised during the construction phase as to the potential for serious segregation of rocks and tire chips. This was expected to be a problem due to a significant difference in the specific gravity of these two very different materials. As a result, it was decided to have the cover stone and the tire chips applied separately.

Once it was decided to have separate applications of cover stone and tire chips, the next issue was the order of application. At the same time, in order to ensure the integrity of the membrane, it was important for the tire chips and hot liquid asphalt spray to form a good bond and develop a matrix. This requirement made it necessary for the tire chips and the asphalt spray to come into contact first. The final component of this membrane was an application of cover stone.

# Membrane Test Strips - First Trial

A test strip of approximately 91.2 meters (300 feet) was constructed to verify various construction parameters. The test strip included a rich asphalt cement spray application at 3.6 liters per square meter (0.8 gallon per square yard) directly on top of the compacted subgrade. This application was followed by an application of 8.1 kilograms per square meter (15 pounds per square yard) of tire chips with a spreader unit in a manner similar to a chip seal application.

The application of tire chips over a freshly applied rich asphalt layer posed a serious construction difficulty. The problem occurred when the hot liquid asphalt migrated upward through the rubber cover chips under pressure from the spreader's tires. This brought the hot liquid asphalt into direct contact with the spreader's tires and caused a "pick-up" problem and ultimately tearing the membrane. It became obvious that the application of tire chips directly on top of the hot asphalt layer was not a feasible option.

# Membrane Test Strips - Second Trial

A second test strip, approximately 91.2 meters (300 feet), was constructed with the application of tire chips first, at the rate of 8.1 kilograms per square meter (15 pounds per square yard) directly on top of the subgrade. There was some concern that the prevailing wind conditions at the construction site may disturb this layer of chips on top of the subgrade; however, this did not prove to be a problem.

The rubber chips were then covered with an asphalt spray at the rate of 3.6 liters per square meter (0.8 gallon per square yard). This proved to be a successful application. The asphalt and tire chips had an opportunity to come into direct contact with each other and form a matrix. Because of affinity of rubber particles for asphalt, some "reaction" was expected to occur on the surface of the tire chips. This membrane was later covered with the cover stone. Total thickness of the finished membrane on this project was approximately 1.25 cm (0.5 in). This is the application that was chosen for the entire project. Finally, the rest of the pavement structure was placed on top of the membrane.

## MEMBRANE MATERIALS

# **Modified Asphalt Cement**

The type of asphalt cement used in the membrane was a polymer modified AC-20 (Kentucky classification: PMAC-1C). The polymer modifier was a styrene-butadiene-styrene (SBS) at the rate of 3% by weight of the asphalt cement. Terry Industries from Ohio supplied the liquid modified asphalt as well as the placement of the tire chips and spraying of the asphalt.

# Tire Rubber Chips

The recycled tires on this project were shredded in such a fashion that produced a gradation distribution; this distribution is presented in Figure 2. The tire chips were free of steel or any other deleterious materials. The chips appeared to develop a "surface reaction" with the hot asphalt spray without any difficulty, which promoted formation of a matrix upon receiving the asphalt spray.

#### Membrane Cover Stone

The run-of-mine (ROM) aggregate material was used to provide a final surface for the membrane prior to other layers of pavement being constructed. The cover stone did an adequate job of binding to the matrix of tire chips and asphalt cement. The gradation of ROM is given in Figure 2.

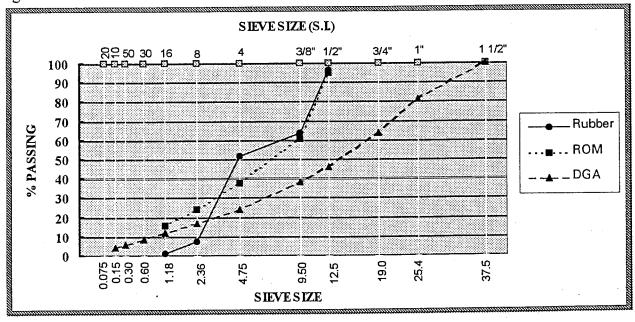


Figure 2. Gradation Chart.

#### Cost of Membrane

The cost of this membrane with rubber chips was  $2.54/m^2$  ( $3.05/yd^2$ ). By comparison, the conventional membrane without the rubber chips would cost  $1.34/m^2$  ( $1.64/yd^2$ ). The additional cost of the membrane which included rubber chips was due to the addition of rubber. Also, the cost of lime stabilization of the subgrade on this project would have been approximately  $50/m^2$  ( $3.00/yd^2$ ). Obviously, these cost figures may vary depending upon the size of the project, location, and other factors. Finally, the tire utilization rate on this project was approximately 1,760 tires per lane mile.

# OWNER/CONTRACTOR INFORMATION

The following is a summary of owner/contractor key contact persons and their respective roles on the construction project.

# Project Management

Kentucky Transportation Cabinet

District-9 Office Flemingsburg, KY

Contacts: (606) 845-2551
Pete Shaffer
Jim Rummage
Paul Ford (Project Engineer)
Stan Martin (Resident Engineer)

#### Prime Contractor

Elmo Greer & Sons, Inc. P.O. Box 730 London, KY 40743

Contacts: (606) 843-6136

Wayne Simmons (project manager)

## Job Description:

Removed existing shoulders, widened cuts, and worked the subgrade to meet the required elevations.

#### Subcontractors:

# 1. Volunteer Highway Supply Company

7603 Maynardville Highway Knoxville, TN 37938

Contacts: (615) 922-7473

# Job Description:

Placed striping on completed project

# 2. H.G. Mays Corporation

P.O. Box 797 Frankfort, KY 40602

Contacts: (502) 875-1282

Tony McGlone Gene Shelton

# Job Description:

Transported and placed all subbase aggregate and rubber chips. Produced, transported, and placed all asphalt mixes. Subcontracted the job of supplying and placing the asphalt membrane to Terry Industries.

## 3. Meeks Electrical, Inc.

700 Northview Paducah, KY 42001

Contacts: (502) 444-7779

## Job Description:

Installation of traffic signals

# 4. Anpat, Inc.

918 South Mayo Trail Pikeville, KY 41501

Contacts: (606) 432-3166

# Job Description:

Placement of guardrail and seeding.

# 5. Terry Industries

8600 Berk Blvd. Hamilton, OH 45015

> Contacts: (513) 874-6192 Todd Terry Mark Terry

# Job Description:

Placed the asphalt membrane on top of the subgrade with and without the rubber chips for the experimental and control sections, respectively.

# PAVEMENT MOISTURE SENSORS

In order to determine the effectiveness of the rubberized asphalt membrane on top of the subgrade, it was decided to install moisture sensors in the subgrade and subbase of the pavement. Details on installation of sensors are given under the section describing the experiment design. These sensors detect the amount of soil moisture based upon soil electrical resistivity. Prior to field installation, in order to ensure accurate characterization of soil moisture content, a detailed calibration of sensors was conducted in the laboratory at the University of Kentucky. Representative soil specimens from the field project were used in this calibration effort. It was discovered that these moisture gages are subjected to a rigorous quality control process by the manufacturer, and that they follow a common pattern for soil moisture content versus resistivity for a given soil. A summary of calibration points and associated regressions are given in Figure 3. Long-term collection of field data on pavement moisture profile needs to be conducted. The construction of this project was completed in mid July of 1995.

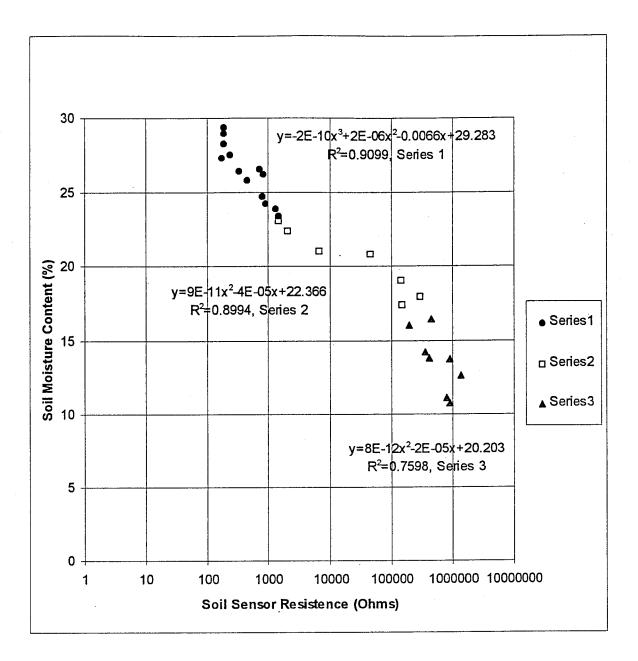


Figure 3. Moisture Gage Calibration Chart.

## PAVEMENT STRUCTURE

# Pavement Subgrade

The subgrade was a clayey material mixed with some shale. This mixture of clay and shale exhibited a CBR of approximately. The Kentucky procedure for "Slake Durability" resulted in a durability index of above 95% for the shale component of the subgrade. In summary, the subgrade was not a particularly strong roadbed material, and it was hypothesized that perhaps it could benefit from chemical stabilization. It was decided to explore the membrane effectiveness as a subgrade moisture stabilizer on this project. To ensure statistically valid comparisons, the eastbound lanes served as the control section (i.e. without membrane), and westbound lanes served as the treatment section (i.e. with membrane).

#### **Pavement Subbase**

The pavement subbase consisted of subgrade was later covered with 38.1 cm (15 in) of a dense graded aggregate (DGA), gradation of which is presented in Figure 2. Generally, DGA bases and subbases are not known to be free draining because of their relatively high fine content. However, in this project we were fortunate to have a DGA subbase with a relatively low fine content.

#### **Pavement Base**

Kentucky Class-I hot mix base material was placed in several lifts: two lifts of 6.98 cm (2.75 in), and a single lift of 6.4 cm (2.5 in); hence, the total asphalt base thickness was 20.3 cm (8 in). This pavement base was later covered with a 3.81-cm (1.5-in) layer of Kentucky Class-I as a binder course prior to application of the surface mix.

#### **Pavement Surface**

Finally, the pavement was finished with 3.17 cm (1.25 in) of Kentucky Class-I surface; this is a conventional dense HMA surface mix in Kentucky. A schematic diagram of the pavement structure is given in Figure 4.

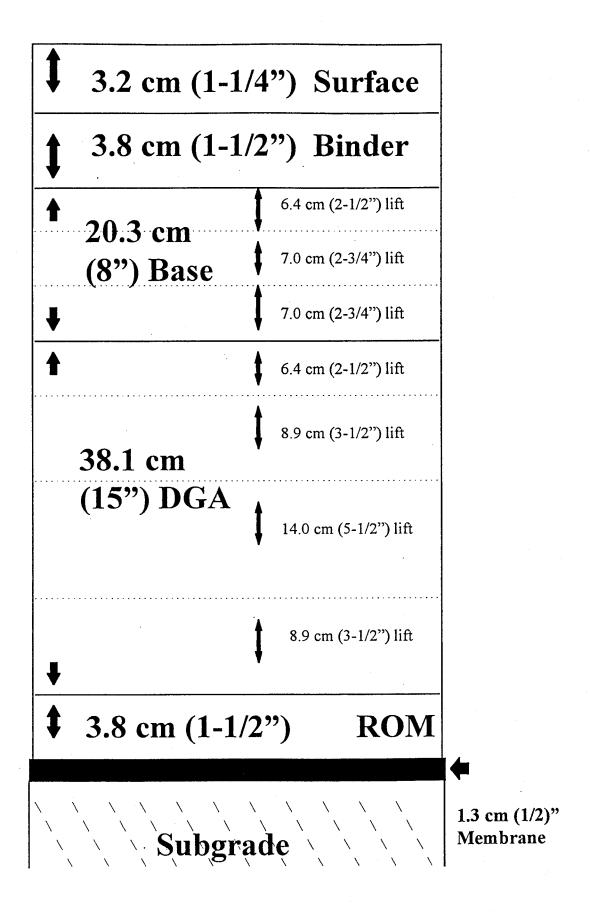


Figure 4. Pavement Structure.

# PAVEMENT MOISTURE DATA

Monthly readings of moisture sensor gages were obtained since the completion of the project (July 1995). These data are presented in Tables 2 and 3, and their graphical representations are given in Figures 5 and 6.

By and large, pavement moisture profiles on this project reflected an unstable trend with a gradual tendency to converge toward a more stable trend. This is to be expected due to initial disturbance of equilibrium conditions of local hydro-geological features at the construction site. Eventually, water flow through the pavement structure is expected to follow a rather stable trend with changes reflecting seasonal and/or climatic variations. However, the hypothesis for moisture equilibrium in a subgrade covered with a rubberized asphalt membrane cannot be supported at this time. In summary, long-term pavement moisture data over a period of several years needs to be collected in order to establish the effectiveness of such a membrane.

		Eastbound	fControl Se	ection	%	::	Westbour	nd-Membra	ine Section	%
Date	Loc.	Temp. (C)	Resistance	Calib. Fact.	Moisture	Loc.	Temp. (C)	Resistance	Calib. Fact.	Moisture
			ohm		***************************************	*!		ohm		:
Jul-95	1t	33.2	445	0.0601	26.76	1t	29.6	765	0.0333	25.49
 !	2t	33.6	60	0.4816	28.89	2t	30.8	335	0.0815	27.30
	3t	33.2	435	0.0616	26.81	3t	29.2	200	0.1402	28.04
	1 b	29.9	780	0.0326	25.45	1b	27.7	750	0.0341	25.54
	2b	30.2	950	0.0263	24.99	2b	26.4	17150	0.0013	21.71
	3b	30.1	755	0.0338	25.53	3b	25.5	370	0.0733	27.12
		<u> </u>	,		***************************************	!		·····	*······	<del>}</del>
Sep-95	1t	31.8	210	0.1333	27.99	1t	33.8	560	0.0469	26.25
.T.:F:	2t	34.2	70	0.4119	28.83	2t	34	510	0.0519	26.46
	3t	34.2	200	0.1402	28.04	3t	33.8	1270	0.0193	24.54
	1b	32.3	580	0.0451	26.17	1b	31.8	870	0.0290	25.19
	2b	32.3	810	0.0313	25.36	2b	32.2	2400	0.0093	22.27
		32.3	610	0.0313	26.05	3b	30.6	1190	0.0207	24.60
	3b	32.2	010	0.0427	20.00	30	30.0	1170		200
Oct-95	1+	14.0	80.0	0.3596	28.77	1t	22.8	180.0	0.1564	28.16
061-93	1t	14.0	······	0.3396	29.09	2t	22.8	100.0	0.2864	28.64
	2t	21.9	30.0	\$		٠ <u></u>	23.0	120.0	0.2377	28.52
	3t	23.0	80.0	0.3596	28.77	3t	23.0	490.0	0.0542	26.55
	1b	22.5	690.0	0.0373	25.75	lb		<b></b>	0.0342	22.29
	2b	22.2	820.0	0.0309	25.33	2b	17.8	2000.0 1475.0	0.0111	24.54
	3b	19.0	510.0	0.0519	26.46	3b	22.0	1473.0	0.0100	21.01
Nov-95	1t	-1.5	60	0.4816	28.89	1t	7.4	115	0.2483	28.55
100-93	2t	5.1	30	0.9696	29.09	2t	7.05	85	0.3381	28.74
	3t	9	60	0.4816	28.89	3t	8.4	50	0.5792	28.96
	1b	8.5	965	0.0259	24.96	1b	9.7	305	0.0900	27.46
	2b	8.45	840	0.0301	25.27	2b	0.7	525	0.0503	26.40
	3b	2.15	975	0.0356	24.93	3b	8.95	1940	0.0115	22.29
		2.10		0.0200	21.70		0.50			:
Jan-96	1t	-7.5	60	0.4816	28.89	1t	2.2	90	0.3189	28.71
0011 70	2t	-0.4	20	1.4576	29.15	2t	2.2	90	0.3189	28.71
	3t	2	60	0.4816	28.89	3t	2.8	50	0.5792	28.96
	lb	2.2	1120	0.0220	24.68	1b	4	300	0.0916	27.49
	2b	2.5	740	0.0220	25.58	2b	<b>-</b> 5	300	0.0916	27.49
	3b	-5.5	1100	0.0225	24.71	3b	3.2	2000	0.0111	22.29
		-0.0	1100	0.0220	4					
Feb-96	1t	-6	40	0.7256	29.02	1t	2	400	0.0674	26.98
FCD-90	2t	0	40	0.7256	29.02	2t	2	60	0.4816	28.89
	3t	2.5	60	0.4816	28.89	3t	3	150	0.1889	28.34
	1b	1.9	1150	0.0214	24.64	1b	2	270	0.1024	27.65
						2b	-7.8	110	0.2598	28.58
	2b	1.8	750	0.0341	25.54	20 3b	1.8	1800	0.0124	22.29
·····	3b	-9	1100	0.0220	24.71	30	1.0	1000		<del></del>
Mar-96	1t	1	50	0.5792	28.96	1t	10	300	0.0916	27.49
	2t	7	20	1.4576	29.15	2t	10	100	0.2864	28.64
	3t	10	200	0.1402	28.04	3t	10	200	0.1402	28.04
		. <u>:</u>				1b	10	300	0.0916	27.49
	lb Ob	9	800	0.0317	25.39	2b	0	200	0.1402	28.04
	2b	-10	400.00	0.0674	26.98	\$ <del>.</del>	8	800	0.0317	25.39
	3b	-2	600	0.0435	26.09	3b		,		

Table 2. Pavement Moisture Data.

		Eastbound	IControl Se	ction	%		Westbou	ndMembra	ne Section	%
Date	Loc.	Temp. (C)	Resistance	Calib Fact	Moisture	Loc.	Temp. (C	Resistance	Calib. Fact	Moisture
		:	ohm					ohm		
						<u>.</u>	<u> </u>			
Apr-96	1t	1	50	0.5792	28.96	1t	10	300	0.0916	27.49
**************************************	2t	7	20	1.4576	29.15	2t	10	100	0.2864	28.64
	3t	10	200	0.1402	28.04	3t	10	200	0.1402	28.04
	1b	9	800	0.0317	25.39	1b	10	300	0.0916	27.49
•	2b	-10	400.00	0.0674	26.98	2b	0	200	0.1402	28.04
	3ъ	-2	600	0.0435	26.09	<b>3</b> b	8	800	0.0317	25.39
May-96	l t	6	20	1.4576	29.15	lt	14	290	0.0950	27.54
May-90	2t	11	10	2.9217	29.22	2t	15	100	0.2864	28.64
	3t	16	10	2.9217	29.22	3t	15	200	0.1402	28.04
	1b	14	780	0.0326	25.45	1b	16	300	0.0916	27.49
	2b	13	400	0.0520	26.98	2b	4	250	0.1110	27.76
				0.0074	25.39	3b	13	800	0.0317	25.39
	3ъ	1	800	0.0317	20.09	30	13		0.0017	
Jun-96	1t	8	10	2.9217	29.22	1t	20	290	0.0950	27.54
	2t	14	10	2.9217	29.22	2t	19	100	0.2864	28.64
	3t	20	20	1.4576	29.15	3t	19	180	0.1564	28.16
	1 b	18	700	0.0367	25.71	1Ъ	20	300	0.0916	27.49
	2b	17	20	1.4576	29.15	2b	7	300	0.0916	27.49
	3b	2	500	0.0530	26.51	3b	16	800	0.0317	25.39
Jul-96	lt	10	10	2.9217	29.22	l t	20	290	0.0950	27.54
5 th-90		16	10	2.9217	29.22	2t	19	100	0.2864	28.64
	2t 3t	25	20	1.4576	29.15	3t	19	180	0.1564	28.16
	at 1b	25 20	700	0.0367	25.71	1b	20	300	0.0916	27.49
				1.4576	29.15	2b	7	300	0.0916	27.49
	2b 3b	19 4	20 500	0.0530	26.51	3b	16	800	0.0317	25.39
	งม		į		20.01				i	

Table 2 (Continued). Pavement Moisture Data.

Moisture Content (%) Westbound Leaving Maysville											
<u>Date</u>	1top	2top	3top	1bott.	2bott.	3bott.					
24.0				20000							
Jul-95	25.49	27.30	28.04	25.54	21.71	27.12					
Sep-95	26.25	26.46	24.54	25.19	22.27	24.60					
Oct-95	28.16	28.64	28.52	26.55	22.29	22.29					
Nov-95	28.55	28.74	28.96	27.46	26.40	22.29					
Jan-96	28.71	28.71	28.96	27.49	27.49	22.29					
Feb-96	26.98	28.89	28.34	27.65	28.58	22.29					
Mar-96	27.49	28.64	28.04	27.49	28.04	25.39					
Apr-96	27.49	28.64	28.04	27.49	28.04	25.39					
May-96	27.54	28.64	28.04	27.49	27.76	25.39					
Jun-96	27.54	28.64	28.16	27.49	27.49	25.39					
Jul-96	27.54	28.64	28.16	27.49	27.49	25.39					
	Moisture	Content (%)	Eastbound -	- Going to M	laysville						
Date	1top	2top	<u>3top</u>	<u>1bott.</u>	2bott.	3bott.					
Jul-95	26.76	28.89	26.81	25.45	24.99	25.53					
Sep-95	27.99	28.83	28.04	26.17	25.36	26.05					
Oct-95	28.77	29.09	28.77	25.75	25.33	26.46					
Nov-95	28.89	29.09	28.89	24.96	25.27	24.93					
Jan-96	28.89	29.15	28.89	24.68	25.58	24.71					
Feb-96	29.02	29.02	28.89	24.64	25.54	24.71					
Mar-96	28.96	29.15	28.04	25.39	26.98	26.09					
Apr-96	28.96	29.15	28.04	25.39	26.98	26.09					
May-96	29.15	29.22	29.22	25.45	26.98	25.39					
Jun-96	29.22	29.22	29.15	25.71	29.15	26.51					
Jul-96	29.22	29.22	29.15	25.71	29.15	26.51					

Table 3. Summary Pavement Moisture Data.

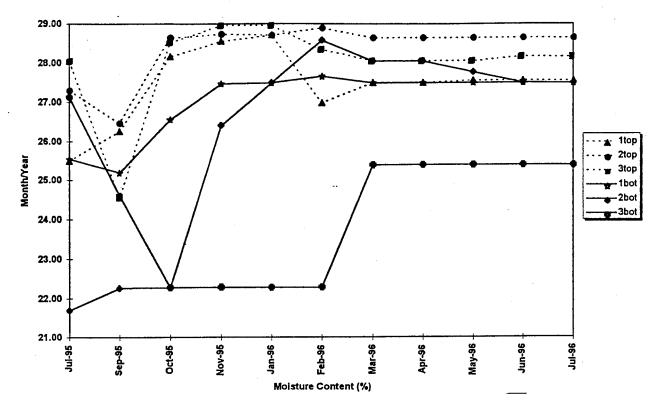


Figure 5. Pavement Moisture Data (Westbound, with Membrane).

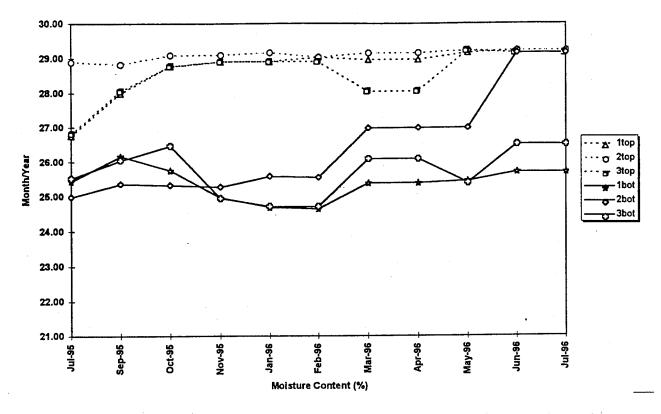


Figure 6. Pavement Moisture Data (Eastbound, without Membrane).

## CONCLUSIONS

- 1. This project demonstrated that scrap tires may be recycled effectively in the form of rubber chips in asphalt interlayer applications.
- 2. Proper construction techniques for the placement of rubber chips in interlayer applications are very important. Kentucky's experience was presented in this report.
- 3. The process presented in this report has the potential of recycling waste tires at an approximate rate of 1,760 tires per lane mile.
- 4. Preliminary pavement moisture profile data are inconclusive. At this time, it is not possible to support the effectiveness of subgrade membrane in allowing the subgrade to achieve moisture equilibrium. Obviously, long-term data are needed in order to effectively characterize the influence of this asphaltic membrane on moisture profile within the pavement structure.
- 5. Long-term performance of the pavement on this project needs to be monitored for evaluating the effects of pavement moisture profile on the overall pavement performance.

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